

Modeling and Forecasting the Peak Flows of a River

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A stochastic model is found for the value of the peak flows of the Mistassibi river in Québec, Canada, when the river is in spate. Next, the objective is to forecast the value of the coming peak flow about four days in advance, when the flow begins to show a marked increase. Both the stochastic model proposed in the paper and a model based on linear regression are used to produce the forecasts. The quality of the forecasts is assessed by considering the standard errors and the peak criterion. The forecasts are much more accurate than those obtained by taking the mean value of the previous peak flows.

Key words: Stochastic modeling; Lognormal distribution; Linear regression; Correlation coefficient; Peak criterion; Hydrology

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1 INTRODUCTION

The problem of forecasting the value of the flow of various rivers and/or hydrological basins in Québec, Canada, has been considered by Labib *et al.* [4] and by the author (see Lefebvre [7], for instance), in particular. Their objective was to forecast the flow up to seven days ahead. They compared the results obtained by making use of various stochastic models to those obtained with a deterministic model known as PREVIS (see Refs. [1, 3, 5, 6]), which is currently used by a number of companies in Canada. It was found that relatively simple stochastic models could outperform PREVIS, which requires the evaluation of 18 parameters, for forecasts up to three and sometimes four days in advance. However, PREVIS generally produces more reliable forecasts from five days ahead.

Next, Lefebvre [8] tried to model the maximum flow of the Mistassibi river during each of the months of April, May and June, as well as to forecast the maximum flow in May, based on the observed maximum flow in April. This three-month period is the time when the river is in spate and the maximum value of the flow in May is also most of the time the maximum flow over the three-month period.

In the present paper, instead of trying to forecast the maximum flow in May, based on the maximum flow in April (which is very often observed on April 30th), we will attempt to forecast the value of the various peaks of the river flow during the period when the river is in spate. In some years, two or even three peaks that could cause flooding were observed. So,

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the problem is different from the one considered in Lefebvre [8]. Moreover, it was found in Lefebvre [8] that the maximum flow in May usually happens around May 15th and that the correlation between the maximum in April and that in May is rather weak. Therefore, it is difficult to make use of the observed maximum flow in April to forecast the maximum flow in May with high accuracy.

Here, we approach the problem of forecasting the peak flows of the Mistassibi river in a different way. More precisely, we will seek to forecast an oncoming peak flow about four days before its occurrence. Indeed, in most cases the river flow shows a marked increase at least three days before a peak. Our objective is to arrive at quite accurate forecasts of these peaks quickly enough so that the persons in charge can take action to prevent flooding if it is deemed necessary.

Other authors have tried to forecast peak flows of rivers, as well as the time of occurrence of these peak flows. Rosbjerg [9], in particular, has proposed a model and an estimator for the maximum flow (see also Ref. [2]). However, Rosbjerg's estimator depends on the correlation coefficient of two consecutive peaks. In our case, there are many years for which there is but a single peak during the whole spring season. Hence, we cannot make use of the formula developed by Rosbjerg.

In Section 2, stochastic models will be found for the peak flows of the Mistassibi river and for the river flows on the previous days. Next, in Section 3 we will make use of the models obtained in Section 2 to forecast the peak flows of the river. As will be seen, even better forecasts will in fact be produced by another model, based on linear regression. Finally, a few remarks will conclude this work in Section 4.

2 STOCHASTIC MODELS

The observed flows of the Mistassibi river are available to us for the period from 1953 to 1994. However, due to numerous missing values for the first years, we decided to limit our study to the years 1963 to 1994. Over this time period, during the months of April, May and June, we have identified 54 occurrences when the river flow has had a daily increase of at least 90 m³/s, leading to a peak flow in the following days. The data are presented in Tables I and II. We have included the value of the flow before the large increase (Flow), the size of the increase (Increase), the value of the flow one and two days after the increase (Flow2 and Flow3), and finally the value of the ensuing peak flow (Max) as well as the number of days elapsed until the peak flow (N). Moreover, Table I presents the data for the years 1963–1979, while Table II does so for the years 1980–1994.

Remark The value of $90 \,\mathrm{m}^3/\mathrm{s}$ was chosen so that the peak flow could be forecasted with enough accuracy and early enough to advise the administrators to take action if needed. A $50 \,\mathrm{m}^3/\mathrm{s}$ increase, for instance, leads to too many "false alarms" or lack of precision, whereas a $150 \,\mathrm{m}^3/\mathrm{s}$ increase as a warning signal would entail missing some peak flows or leaving too little time to the administrators.

First, we find that the peak flows occurred on average approximately 3.5 days after the $90\,\mathrm{m}^3/\mathrm{s}$ (or more) increase. Therefore, if we could produce accurate enough forecasts of the peak flows when this large increase is observed, it would leave a few days to act in order to prevent floodings.

Next, we have tested the hypothesis that the variables Flow, Flow2, Flow3 and Max in Tables I and II combined follow a Gaussian distribution, as well as the variable

TABLE I Data for the years 1963-1979.

| Date | Flow | Increase | Flow2 | Flow3 | Max | N |
|----------|------|----------|-------|-------|------|----|
| 63/05/19 | 660 | 150 | 949 | 971 | 971 | 3 |
| 64/05/01 | 510 | 136 | 745 | 801 | 1240 | 11 |
| 65/05/09 | 220 | 131 | 487 | 575 | 728 | 5 |
| 65/05/17 | 731 | 161 | 1000 | 1030 | 1030 | 3 |
| 66/05/17 | 219 | 109 | 459 | 711 | 1010 | 6 |
| 67/05/27 | 425 | 105 | 617 | 674 | 668 | 4 |
| 68/04/22 | 411 | 99 | 600 | 711 | 1000 | 5 |
| 69/05/18 | 580 | 97 | 750 | 818 | 818 | 3 |
| 69/06/04 | 663 | 96 | 799 | 793 | 813 | 4 |
| 70/05/01 | 382 | 170 | 878 | 997 | 997 | 3 |
| 70/05/17 | 430 | 97 | 682 | 714 | 739 | 4 |
| 70/06/11 | 402 | 139 | 651 | 595 | 651 | 2 |
| 71/05/10 | 597 | 94 | 756 | 841 | 960 | 5 |
| 72/05/15 | 268 | 97 | 515 | 671 | 1060 | 7 |
| 73/04/24 | 300 | 91 | 467 | 504 | 748 | 7 |
| 73/05/04 | 773 | 164 | 1050 | 1030 | 1050 | 2 |
| 74/05/12 | 408 | 158 | 733 | 892 | 1030 | 6 |
| 74/06/01 | 1080 | 130 | 1300 | 1320 | 1350 | 5 |
| 75/05/04 | 272 | 96 | 411 | 476 | 762 | 9 |
| 75/06/01 | 405 | 110 | 578 | 561 | 578 | 2 |
| 76/04/29 | 484 | 99 | 614 | 674 | 1010 | 6 |
| 76/05/12 | 685 | 300 | 1250 | 1300 | 1300 | 3 |
| 76/05/18 | 1350 | 130 | 1530 | 1560 | 1560 | 3 |
| 77/04/24 | 125 | 97 | 419 | 504 | 544 | 5 |
| 77/05/07 | 580 | 100 | 680 | 629 | 680 | 1 |
| 77/05/17 | 782 | 113 | 991 | 1000 | 1000 | 3 |
| 77/05/23 | 997 | 93 | 1150 | 1150 | 1150 | 2 |
| 78/05/09 | 438 | 136 | 757 | 988 | 1160 | 8 |
| 78/06/13 | 417 | 336 | 985 | 1070 | 1070 | 3 |
| 79/04/27 | 449 | 285 | 1090 | 1380 | 1480 | 4 |
| 79/05/13 | 584 | 149 | 896 | 862 | 896 | 2 |
| 79/06/12 | 448 | 100 | 575 | 573 | 701 | 7 |

TABLE II Data for the years 1980-1994.

| Date | Flow | Increase | Flow2 | Flow3 | Max | N |
|----------|------|----------|-------|-------|------|----|
| 80/04/25 | 246 | 121 | 419 | 489 | 911 | 9 |
| 81/05/06 | 437 | 105 | 621 | 614 | 621 | 2 |
| 81/05/14 | 853 | 132 | 1130 | 1170 | 1230 | 4 |
| 82/05/07 | 550 | 133 | 871 | 1050 | 1200 | 5 |
| 82/06/01 | 391 | 116 | 685 | 689 | 689 | 3 |
| 83/04/30 | 600 | 163 | 905 | 939 | 961 | 5 |
| 83/05/14 | 584 | 135 | 882 | 960 | 960 | 3 |
| 84/04/25 | 321 | 100 | 515 | 583 | 986 | 8 |
| 85/05/17 | 528 | 134 | 837 | 950 | 1100 | 5 |
| 85/06/02 | 696 | 132 | 826 | 763 | 826 | 1 |
| 86/04/27 | 493 | 158 | 857 | 1010 | 1290 | 7 |
| 87/04/02 | 352 | 97 | 414 | 373 | 449 | 2 |
| 87/04/19 | 361 | 90 | 504 | 530 | 580 | 5 |
| 89/05/01 | 115 | 96 | 250 | 289 | 854 | 10 |
| 90/05/11 | 635 | 130 | 884 | 833 | 884 | 2 |
| 91/05/01 | 437 | 110 | 654 | 691 | 691 | 3 |
| 92/05/09 | 422 | 124 | 665 | 720 | 1280 | 6 |
| 92/05/18 | 1170 | 170 | 1380 | 1320 | 1380 | 2 |
| 93/04/13 | 188 | 98 | 340 | 302 | 616 | 7 |
| 93/05/04 | 528 | 159 | 979 | 1250 | 1300 | 4 |
| 93/06/02 | 531 | 102 | 692 | 660 | 692 | 2 |
| 94/05/06 | 284 | 143 | 591 | 646 | 848 | 8 |

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TABLE III *p*-Values associated with the normality tests.

| p-Value | | | |
|---------|--|--|--|
| 0.003 | | | |
| 0.023 | | | |
| 0.358 | | | |
| 0.212 | | | |
| 0.410 | | | |
| | | | |

TABLE IV p-Values associated with the normality tests applied to the logarithms of the variables in Table III.

| Variable | p-Value |
|----------|---------|
| LnFlow | 0.153 |
| LnFlow1 | 0.490 |
| LnFlow2 | 0.892 |
| LnFlow3 | 0.507 |
| LnMax | 0.433 |

we have used the Anderson-Darling test (as well as the Ryan-Joiner test actually). The p-values of the tests, that is, the smallest values of the level α of the tests that can be used to reject normality, are shown in Table III.

We see that, apart from the variables Flow and Flow1, the normality assumption cannot be rejected with a small α . However, we have also applied the Anderson-Darling test to the natural logarithms of the same variables. The corresponding p-values are given in Table IV.

It is obvious that the lognormal distribution is a better model than the Gaussian distribution for the data.

Then, we have computed the correlation coefficient of LnMax and each of the variables LnFlow, LnFlow1, LnFlow2 and LnFlow3 (see Tab. V).

As could be expected, the correlation coefficient of LnMax and the natural logarithm of the observed flow increases when the observed flow is closer to the maximum.

In the next section, the various Flow variables will be used to try to forecast the peak flows as accurately as possible. For the moment, we are looking for a stochastic model for the peak flows. Based on what precedes, we can state that the *natural logarithm* of the peak flow seems to follow a *Gaussian* distribution with mean and standard deviation approximately equal to 6.8148 and 0.2814. Similarly, the variables LnFlow, LnFlow1, LnFlow2 and LnFlow3 also seem to have a Gaussian distribution with means and standard deviations given in Table VI.

TABLE V Correlation coefficients of LnMax and the logarithms of the flows.

| Variable | Correlation coefficient | | |
|----------|-------------------------|--|--|
| LnFlow | 0.505 | | |
| LnFlow1 | 0.578 | | |
| LnFlow2 | 0.669 | | |
| LnFlow3 | 0.782 | | |

| TABLE VI | Means and standard deviations | of |
|---------------|-------------------------------|----|
| the variables | LnFlow, LnFlow1, LnFlow2 a | nd |
| LnFlow3. | | |

| Variable | Mean | Standard deviation |
|----------|--------|--------------------|
| LnFlow | 6.1313 | 0.4949 |
| LnFlow1 | 6.3936 | 0.4058 |
| LnFlow2 | 6.5744 | 0.3704 |
| LnFlow3 | 6.6467 | 0.3657 |

Finally, given that the Gaussian distribution is a good model for the variable LnMax, it is a simple matter to compute estimates of the flow values that the peak flow exceeds with probability 1% or 0.1%, etc.

3 FORECASTING THE PEAK FLOW VALUES

We now turn to the problem of forecasting the value of the peak flow, based on the observed flow, together with the value of the 90^+ m 3 /s increase. Moreover, because the peak flow actually occurs, on average, about 3.5 days after the large flow increase has been noticed, it is interesting to try to forecast the value of the peak flow once the actual flows one and even two days after this large flow increase are known. We will make use of the data in Table I, that is for the years 1963–1979, to arrive at an estimator, and then we will attempt to forecast the peak flows that have been observed over the years 1980 to 1994.

A first formula can be obtained by remembering that if the random vector (X_1, X_2) has a bivariate Gaussian distribution, then

$$E[X_2|X_1=x] = \mu_{X_2} + \rho_{X_1,X_2} \frac{\sigma_{X_2}}{\sigma_{X_1}} (x - \mu_{X_1}),$$

where μ_{X_i} is the mean of X_i , σ_{X_i} is its standard deviation and ρ_{X_1,X_2} is the correlation coefficient of X_1 and X_2 . Hence, computing the various quantities needed to estimate the mean of the variable Max, based on the value of the variable Flow, we obtain the following formula (see Tabs. VII and VIII):

Gaus :=
$$\exp\left\{6.8344 + 0.556\left(\frac{0.2652}{0.4932}\right) (\text{LnFlow} - 6.1674)\right\}$$
.

TABLE VII Means and standard deviations of the variables LnFlow, LnFlow1, LnFlow2, LnFlow3 and LnMax using the data in Table I.

| Variable | Mean | Standard deviation |
|----------|--------|--------------------|
| LnFlow | 6.1674 | 0.4932 |
| LnFlow1 | 6.4294 | 0.4075 |
| LnFlow2 | 6.6181 | 0.3435 |
| LnFlow3 | 6.7053 | 0.3136 |
| LnMax | 6.8344 | 0.2652 |

| TABLE | VII | 1 (| Correlation | coef | ficie | ents of |
|--------------|------|-------|-------------|------|-------|---------|
| LnMax | and | the | logarithms | of | the | flows |
| using th | e da | ta in | Table I. | | | |

| Variable | Correlation coefficient | | |
|----------|-------------------------|--|--|
| LnFlow | 0.556 | | |
| LnFlow1 | 0.635 | | |
| LnFlow2 | 0.718 | | |
| LnFlow3 | 0.853 | | |

Similarly, the forecasts based on LnFlow1 and on the observed flows one and two days after the 90⁺ m³/s increase are respectively

Gaus1 :=
$$\exp\left\{6.8344 + 0.635 \left(\frac{0.2652}{0.4075}\right) (\text{LnFlow1} - 6.4294)\right\},\$$

Gaus2 := $\exp\left\{6.8344 + 0.718 \left(\frac{0.2652}{0.3435}\right) (\text{LnFlow2} - 6.6181)\right\},\$

and

Gaus3 :=
$$\exp\left\{6.8344 + 0.853 \left(\frac{0.2652}{0.3136}\right) (\text{LnFlow3} - 6.7053)\right\}$$
.

To assess the quality of the forecasts produced by the estimators above, we will compute the correlation coefficients of the forecasts and the observed peak flows. However, two more important criteria are the standard error defined by

$$STD = \left\lceil \frac{SSQ}{n-1} \right\rceil^{1/2},$$

where SSQ is the sum of the squares of the forecasting errors and n = 22 in our case, and the peak criterion:

$$PC := \frac{\left[\sum_{i=1}^{22} (\hat{X}_k - X_k)^2 X_k^2\right]^{1/4}}{\left[\sum_{i=1}^{22} X_k^2\right]^{1/2}},\tag{1}$$

in which X_k is the kth observed peak flow and \hat{X}_k is the corresponding forecasted peak flow. This last criterion enables us to measure the capacity of the estimator for forecasting very high flows, which is really important. Moreover, we can show that the peak criterion PC is in the interval [0,1] and that the closer to 0 the quantity PC is, the better the forecasts are.

Next, to get a better idea of the quality of the forecasts produced by the formulae Gaus, Gaus1, etc., we will also compute the values of the criteria above obtained with other estimators. A "naive" estimator of the next peak flow is the average value of all the previous peak flows observed. Therefore, we can estimate the first peak flow in Table II by computing the average value of the 32 peak flows in Table I; then, we estimate the second peak flow in Table II by computing the mean of the 33 previous peak flows, etc. We denote this estimator by MEAN.

Since the maximum flow occurs on average between four and five days after the variable Flow has been observed, another simple estimator of the peak flow is given by

Remarks (1) Notice that this estimator doesn't make use of the data in Table I. (2) Because the mean value of the variable N is approximately equal to 4.6, we could have set LIN1 = Flow $+4.6 \times$ Increase instead. However, as will be seen below, this estimator isn't very reliable and its performance is actually worse when we replace 5 by 4.6.

In a similar way, once the values of Flow2 and Flow3 are known, we can estimate the coming peak flow by

$$LIN2 := Flow1 + 4 \times (Flow2 - Flow1)$$

and

LIN3 := Flow2 +
$$3 \times$$
 (Flow3 - Flow2).

Finally, a technique that has been used successfully by the author in previous papers is that of linear regression. The various regression equations obtained with the data in Table I are the following:

$$Max = 414 + 0.550 \times Flow + 1.86 \times Increase := Reg1,$$

$$Max = 368 - 0.599 \times Flow - 0.69 \times Increase + 1.27 \times Flow2 := Reg2$$

and

$$Max = 248 + 0.724 \times Flow + 0.983 \times Increase - 1.85 \times Flow2 + 1.93 \times Flow3 := Reg3.$$

Remark These regression equations are quite reliable. Indeed, the corresponding coefficients of determination \mathbb{R}^2 are given by 55.3%, 62.0% and 85.7% respectively. Moreover, if we include the variable \mathbb{N} into the model, we get a coefficient of determination (approximately) equal to 92.9%. Of course, the value of \mathbb{N} is not known when we want to forecast the next peak flow, so it cannot be used in the forecasting formulae.

First, Table IX presents the correlation coefficients of Max and the various estimators proposed above.

We see that the LIN and Reg estimators have the best correlation coefficients, the Gaus estimators being a little less correlated with the variable Max. Although the important criteria are the standard error and the peak criterion, as mentioned above, we may immediately conclude that the MEAN estimator is really not reliable at all.

Next, we compute the standard errors obtained with each of the estimators in Table IX. The results are shown in Table X.

TABLE IX Correlation coefficients of Max and its various estimators.

| Estimator | Correlation coefficient | |
|-----------|-------------------------|--|
| MEAN | -0,389 | |
| LIN1 | 0.666 | |
| LIN2 | 0.749 | |
| LIN3 | 0.825 | |
| Gaus | 0.498 | |
| Gaus1 | 0.555 | |
| Gaus2 | 0.667 | |
| Gaus3 | 0.779 | |
| Reg1 | 0.638 | |
| Reg2 | 0.734 | |
| Reg3 | 0.837 | |

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TABLE X Standard errors (in m³/s) obtained with the various estimators of Max.

| Estimator | Standard error | | |
|-----------|----------------|--|--|
| MEAN | 274.8 | | |
| LIN1 | 310.7 | | |
| LIN2 | 310.3 | | |
| LIN3 | 239.2 | | |
| Gaus | 234.6 | | |
| Gaus1 | 225.4 | | |
| Gaus2 | 203.6 | | |
| Gaus3 | 180.9 | | |
| Reg1 | 208.9 | | |
| Reg2 | 184.2 | | |
| Reg3 | 162.8 | | |

TABLE XI Peak criteria obtained with various estimators of Max.

| Estimator | Peak criterion |
|-----------|----------------|
| MEAN | 0.2451 |
| Gaus | 0.2611 |
| Gaus1 | 0.2254 |
| Gaus2 | 0.2632 |
| Gaus3 | 0.2378 |
| Reg1 | 0.2184 |
| Reg2 | 0.2040 |
| Reg3 | 0.1977 |

Table X enables us to discard the LIN estimators, since the corresponding Gaus and Reg estimators are clearly better.

Finally, another important criterion is the peak criterion defined in (1). We computed this criterion for the estimators MEAN, Gaus and Reg, as shown in Table XI.

Surprisingly, the naive estimator MEAN has a smaller value of PC than Gaus and Gaus 2. At any rate, it is now obvious that the best forecasts are produced by the Reg estimators. Indeed, the Reg estimators always have the smallest standard errors and the smallest values of the peak criterion. We see that, compared to the accuracy of the MEAN estimator, there is a decrease of 24% to over 42% in the standard error.

The Gaus estimators do quite well as far as the standard error is concerned; however, they don't seem to be able to forecast (very) high flows very well, which is a serious flaw. Notice that these estimators are all based on a bivariate Gaussian distribution, whereas Regi involves i+1 variables, for i=1,2,3.

4 CONCLUDING REMARKS

In this paper, a simple stochastic model was first proposed for the peak flows of the Mistassibi river, in Québec, during the months of April to June, that is, when the river is in spate. We saw in Section 2 that the variable Max seems to have a lognormal distribution (although a Gaussian distribution would also have been acceptable for this variable). This model enables us to

compute flow values that will only be exceeded with a given (small) probability. Confidence intervals could also be obtained from this model for the mean peak flow.

Next, we turned to the problem of forecasting the peak flows of the river once an important increase in the daily flow has been observed. We found in Section 3 that a very good estimator of the coming peak flow is obtained by making use of linear regression. Of course, the more observations of the actual flow we have, the more accurate the forecasts are. Since the peak flow occurs on average approximately three and a half days after an increase of 90 or more m³/s, we considered the estimators based on the flows before and after the large increase, as well as the flows one and two days after the large increase. Although the estimator based on all these variables at the same time is naturally the most accurate, it might often be too late to act if we wait two days after having noticed a large increase in the daily flow. Therefore, the other estimators (Reg1 in particular) are also useful. The administrators can always react and change their decision if the forecast produced by Reg2 is less alarming than that provided by Reg1, for instance.

Now, to improve further the accuracy of the forecasts, we could try to incorporate at least another variable into the regression equation, such as the amount of precipitation on the day when the large flow increase was observed. This was done in Lefebvre [8] without much success; however, as mentioned previously, there is not enough correlation between the maximum flow in April and that in May. In the present case, knowing the amount of precipitation a few days before the peak flow could prove to be useful.

Finally, it has been found in other works that another way of improving the forecasts is to take the arithmetic mean of the forecasts produced by various estimators. Here, if we define

$$Ave 3 = \frac{MEAN + LIN3 + Gaus 3 + Reg 3}{4},$$

we find that the correlation coefficient of Max and Ave3 is 0.827, the standard error computed with this estimator is 156.1 m³/s, and the peak criterion is 0.1951. These values of the standard error and of the peak criterion are better than those obtained with any single estimator. So, even if some estimators (such as MEAN) considered individually do not provide reliable forecasts, they can nevertheless be used to improve the quality of the forecasts. Actually, if we define instead

$$Ave3 = \frac{LIN3 + Gaus3 + Reg3}{3},$$

we find that the correlation coefficient of Max and Ave3 increases to 0.829; however, the standard error and the peak criterion also increase, to 177.6 m³/s and 0.2042 respectively. Thus, rather surprisingly, it is better here to leave MEAN in the formula.

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